



Developing Rainfall Intensity-Duration-Frequency Relationship for Central Zone of Surat City, Gujarat, India

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Abstract—Intensity–Duration–Frequency (IDF) relationship of rainfall amounts is one of the most commonly used tools in water resources engineering for planning, design and operation of water resources projects. Historical rainfall records for the most of towns and urban areas in India are available only from non–recording rain gauges. These are measurements of daily/24 hours storm depths. The objective of this study is to derive IDF relationship of rainfall at central zone of Surat city from daily/24 hours rainfall data. These relationships are useful in the design of urban drainage works, e.g. storm sewers, culverts and other hydraulic structures. Gumbel distribution and the Log Pearson Type III distribution (LPTIII) techniques are commonly used to develop the IDF curve from rainfall data. In the present study Gumbel distribution method has been used to derive IDF curve.

Keywords: IDF curve, Rainfall intensity, Rainfall duration, Rainfall frequency relationships.

I. INTRODUCTION

Rainfall intensity–duration–frequency IDF curves are graphical representations of the amount of water that falls within a given period of time in catchment areas (Dupont and Allen, 2000). IDF curves are used to aid the engineers while designing urban drainage works. The establishment of such relationships was done as early as 1932 (Chow (1988) and Dupont and Allen (2006)). Since then, many sets of relationships have been constructed for several parts of the globe. In some study IDF curves have been used in conjunction with runoff estimation formulae such as Rational Method, to predict the peak runoff amounts from a particular watershed. The information from the curves is then used in hydraulic design to size culverts and pipes (Dupont and Allen, 2000). Further studies by Ilona and France's (2002) performed rainfall analysis and regionalization of IDF curves for different regions. AlHassoun (2011) developed an empirical formula to estimate the rainfall intensity in Riyadh region. He found that there is not much difference in the results of rainfall analysis of IDF curves in Riyadh area between Gumbel and LPT III methods. He attributed this to the fact that Riyadh region has semi-arid climate and flat topography. In the lower reaches of Tapi River topography is almost flat similar to Riyadh region. Therefore, in this study we have used Gumbel distribution method.

II. STUDY AREA

Surat City (Latitude 21.1702°N and longitude 72.8311°E) is situated on bank of Tapi River is having flat topography. Coastline of Arabian Sea is at a distance about 19.4 km from the study area. Area of Surat city is 326.515 sq.km and population 44,66,826 (Census 2011). Surat city is divided in seven main zones namely Central zone, East zone, West zone, North zone, South zone, South east zone, Southwest zone. The study area is located in Central zone of Surat which is divided into 12 main wards having 19513 meters of storm water drainage network.

The summers are quite hot with temperatures ranging from 37.78 C to 44.44 C. The climate is pleasant during the monsoon while autumn is temperate. The winters are not very cold but the temperatures in January range from 10 C to 15.5 C. The average annual rainfall of the city has been 1143 mm

III. DATA SOURCE

IRS P6 LISS-IV satellite image were used for the preparation of land-use/land-cover map of study area. Soil map and index map was collected from Bhaskaracharya Institute for Space Application and Geo-Informatics (BISAG), Gandhinagar, Gujarat. Daily rainfall data of 6 rain-gauge station were collected from the State Water Data Centre (SWDC), Gandhinagar, Gujarat. ArcGIS 9.3 is used at BISAG, Gandhinagar, Gujarat.



Fig.1: Location map of the study Area

IV. Methodology and Analysis of data

Methodology of the study has been given as flow chart in Fig.2. Based on the hydrological data obtained from SWDC and generated using GIS tools have been analyzed for the development of IDF curve for different return period.

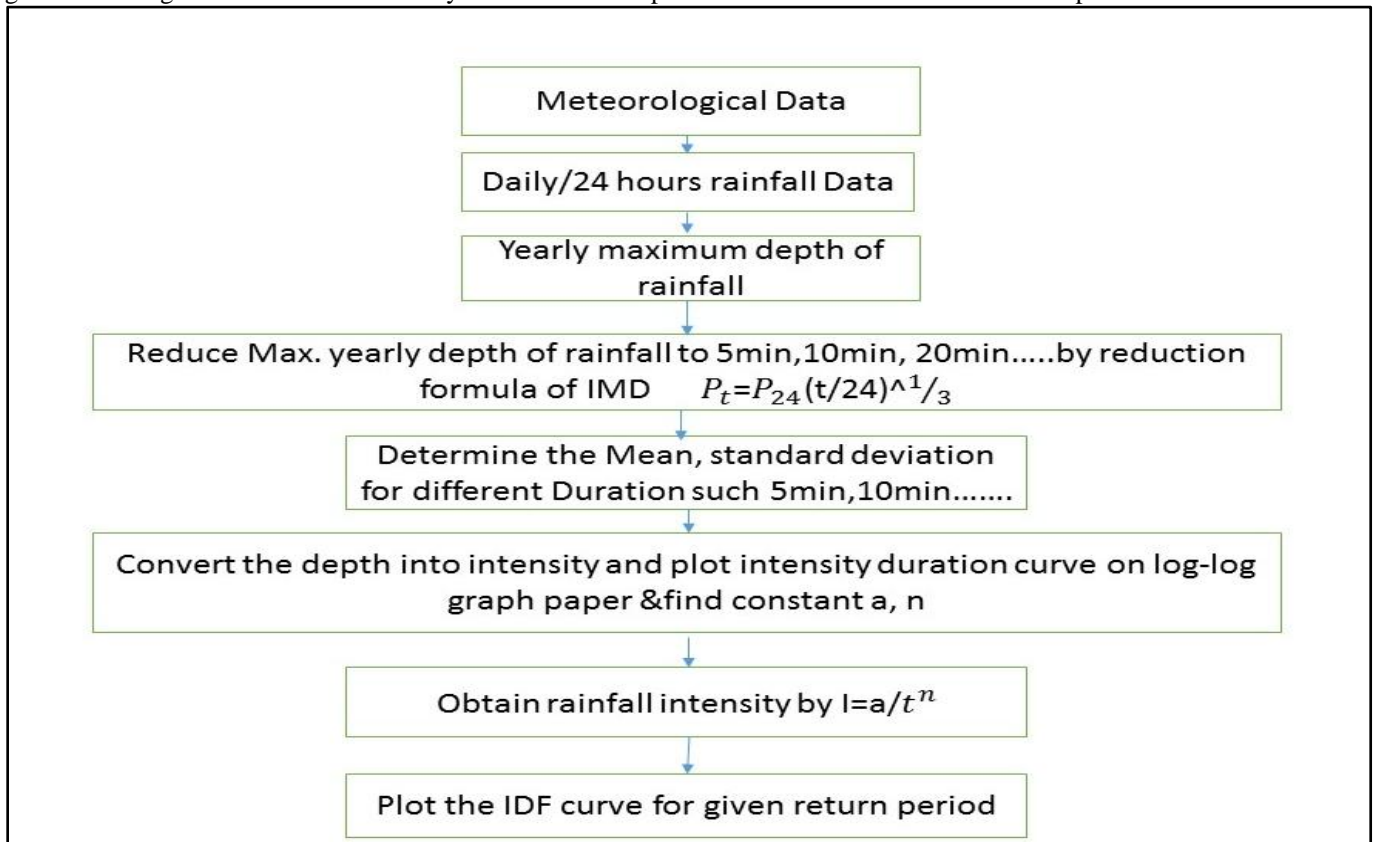


Fig. 2: Methodology

V. GUMBEL DISTRIBUTION METHOD

Gumbel theory of distribution:

Gumbel distribution methodology was selected to perform the flood probability analysis. The Gumbel theory of distribution is the most widely used distribution for IDF analysis owing to

Its suitability for modelling maxima. It is relatively simple and uses only extreme events (maximum values or peak rainfalls). The Gumbel method calculates the 2, 5, 10, 25, 50 and 100-

Year return intervals for each duration period and requires several calculations. Frequency precipitation P_T (in mm) for each duration with a specified return period T (in year) is given by

The following equation.

$$P_T = P_{ave} + KS$$

Where K is Gumbel frequency factor given by:

$$K = -\frac{\sqrt{6}}{\pi} \left[0.5772 + \ln \left[\ln \left[\frac{T}{T-1} \right] \right] \right]$$

Where P_{ave} is the average of the maximum precipitation corresponding to a specific duration.

In utilizing Gumbel's distribution, the arithmetic average in Eq. (1) is used:

$$P_{ave} = \frac{1}{n} \sum_{i=1}^n P_i$$

Where P_i is the individual extreme value of rainfall and n is the number of events or years of record. The standard deviation is calculated by Eq. (4) computed using the following

Relation:

$$S = \left[\frac{1}{n-1} \sum_{i=1}^n (P_i - P_{ave})^2 \right]^{1/2}$$

Where S is the standard deviation of P data. The frequency factor (K), which is a function of the return period and sample size, when multiplied by the standard deviation gives the

Departure of a desired return period rainfall from the average. Then the rainfall intensity, I (in mm/h) for return period T is Obtained from:

$$I_t = \frac{P_t}{T_d}$$

Where T_d is duration in hours. The frequency of the rainfall is usually defined by reference to the annual maximum series, which consists of the largest values observed in each year. An alternative data format for rainfall frequency studies is that based on the peak-over threshold concept, which consists of all precipitation amounts

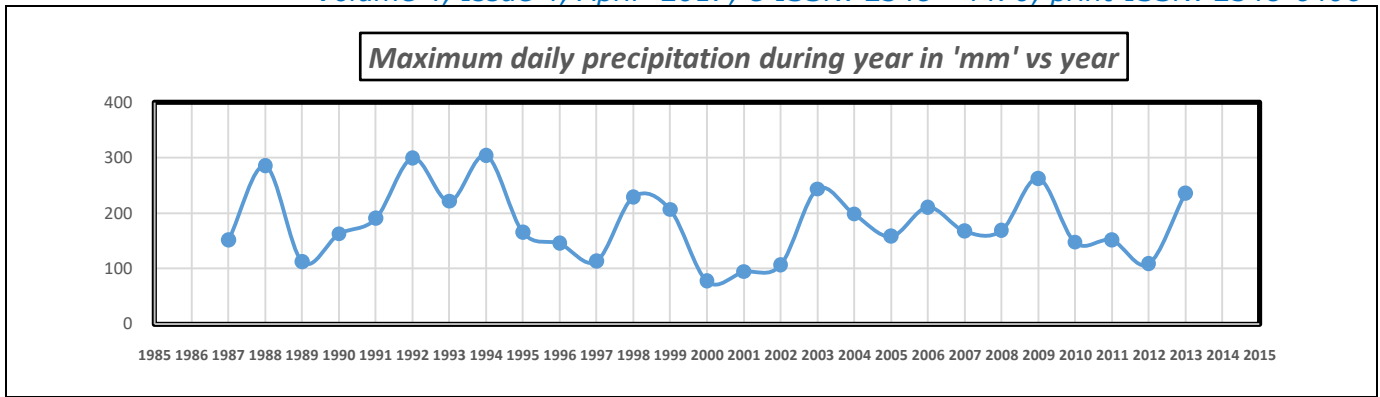
Above certain thresholds selected for different durations. Due to its simpler structure, the annual-maximum-series method is more popular in practice (Borga, Vezzani and Fontana, 2005). From the raw data, the maximum precipitation (P) and the statistical variables (average and standard deviation) for each duration (10, 20, 30, 60, 120, 180, 360, 720, 1440 min) were computed. Tables 1 and 2 show the computed frequency precipitation (P_T) values and intensities (I_T) for different

VI. DEVELOPING OF IDF CURVES FROM DAILY/24 HOURS RAINFALL DATA

Daily rainfall data collected from SWDC (state water data center), Gandhinagar for 28 years

TABLE-1: Yearly Max Depth mm

<i>Year</i>	<i>Maximum daily precipitation during year in 'mm'</i>
<i>1985</i>	<i>165</i>
<i>1986</i>	<i>93</i>
<i>1987</i>	<i>152</i>
<i>1988</i>	<i>286</i>
<i>1989</i>	<i>113</i>
<i>1990</i>	<i>163</i>
<i>1991</i>	<i>191</i>
<i>1992</i>	<i>300</i>
<i>1993</i>	<i>222</i>
<i>1994</i>	<i>305</i>
<i>1995</i>	<i>166</i>
<i>1996</i>	<i>145.8</i>
<i>1997</i>	<i>114</i>
<i>1998</i>	<i>230</i>
<i>1999</i>	<i>207</i>
<i>2000</i>	<i>77.8</i>
<i>2001</i>	<i>94.6</i>
<i>2002</i>	<i>107</i>
<i>2003</i>	<i>244</i>
<i>2004</i>	<i>199</i>
<i>2005</i>	<i>158.6</i>
<i>2006</i>	<i>211.4</i>
<i>2007</i>	<i>168.2</i>
<i>2008</i>	<i>169.2</i>
<i>2009</i>	<i>263.6</i>
<i>2010</i>	<i>148</i>
<i>2011</i>	<i>152</i>
<i>2012</i>	<i>109</i>
<i>2013</i>	<i>237</i>



Reduced each of the maximum 24 hours depth series to 5min, 10min, 15min,20min, from reduction formula by IDM given below:

$$P_t = P_{24} \left(\frac{t}{24} \right)^{1/3}$$

P_t = Required precipitation depth for t hours in mm, P_{24} = Daily precipitation in mm, t = time duration in hours for which the rainfall depth is required

sr. no	Year	daily precipitation during year in 'mm'	$P_t = P_{24} \left(\frac{t}{24}\right)^{1/3}$							(Pi-Pavg) ²						
			5min(0.083 hr)	10min(0.166 hr)	20min(0.333 hr)	30min(0.5 hr)	40min(0.66 hr)	50min(0.833 hr)	60min(1hr)	5min(0.083 hr)	10min(0.166 hr)	20min(0.333 hr)	30min(0.5 hr)	40min(0.66 hr)	50min(0.833 hr)	60min(1hr)
1	1985	165	25.43	31.96	40.22	45.99	50.40	54.43	57.81	4.67	7.38	11.71	15.28	18.37	21.45	24.19
2	1986	93	14.33	18.02	22.67	25.92	28.41	30.68	32.58	175.77	277.70	439.81	574.93	690.67	805.54	908.74
3	1987	152	23.42	29.44	37.05	42.37	46.43	50.14	53.26	17.35	27.41	43.44	56.74	68.18	79.56	89.74
4	1988	286	44.07	55.40	69.71	79.72	87.37	94.34	100.21	271.75	429.44	679.71	889.11	1067.77	1244.88	1404.50
5	1989	113	17.41	21.89	27.54	31.50	34.52	37.28	39.59	103.55	163.59	259.11	338.67	406.87	474.57	535.36
6	1990	163	25.12	31.58	39.73	45.43	49.79	53.77	57.11	6.10	9.64	15.28	19.95	23.98	28.00	31.58
7	1991	191	29.43	37.00	46.56	53.24	58.35	63.01	66.92	3.40	5.38	8.50	11.14	13.37	15.56	17.57
8	1992	300	46.23	58.11	73.12	83.62	91.64	98.96	105.11	347.54	549.19	869.29	1137.06	1365.55	1592.10	1796.22
9	1993	222	34.21	43.00	54.11	61.88	67.82	73.23	77.78	43.85	69.31	109.65	143.50	172.29	200.82	226.59
10	1994	305	47.00	59.08	74.34	85.01	93.17	100.61	106.86	376.87	595.53	942.64	1232.99	1480.77	1726.44	1947.78
11	1995	166	25.58	32.16	40.46	46.27	50.71	54.76	58.16	4.03	6.37	10.10	13.18	15.85	18.50	20.87
12	1996	145.8	22.47	28.24	35.54	40.64	44.54	48.10	51.08	26.22	41.42	65.64	85.76	103.05	120.23	135.62
13	1997	114	17.57	22.08	27.79	31.78	34.82	37.61	39.94	100.43	158.67	251.32	328.49	394.64	460.31	519.27
14	1998	230	35.44	44.55	56.06	64.11	70.26	75.87	80.59	61.70	97.51	154.29	201.89	242.42	282.58	318.83
15	1999	207	31.90	40.10	50.46	57.70	63.23	68.28	72.53	18.58	29.37	46.45	60.81	73.00	85.07	95.98
16	2000	77.8	11.99	15.07	18.96	21.69	23.77	25.66	27.26	243.37	384.51	608.93	796.06	956.28	1115.30	1258.19
17	2001	94.6	14.58	18.33	23.06	26.37	28.90	31.21	33.15	169.30	267.47	423.60	553.74	665.22	775.86	875.26
18	2002	107	16.49	20.73	26.08	29.82	32.69	35.30	37.49	123.22	194.67	308.33	403.02	484.17	564.72	637.06
19	2003	244	37.60	47.27	59.47	68.01	74.54	80.49	85.49	100.25	158.43	250.71	328.01	393.89	459.17	518.06
20	2004	199	30.67	38.55	48.51	55.47	60.79	65.64	69.72	9.47	14.97	23.67	31.00	37.21	43.35	48.92
21	2005	158.6	24.44	30.72	38.66	44.21	48.45	52.32	55.57	9.91	15.65	24.82	32.41	38.95	45.46	51.28
22	2006	211.4	32.58	40.95	51.53	58.92	64.58	69.73	74.07	24.89	39.33	62.22	81.44	97.77	113.95	128.57
23	2007	168.2	25.92	32.58	41.00	46.88	51.38	55.48	58.93	2.79	4.40	6.98	9.10	10.95	12.79	14.42
24	2008	169.2	26.08	32.78	41.24	47.16	51.69	55.81	59.28	2.29	3.62	5.75	7.50	9.02	10.54	11.88
25	2009	263.6	40.62	51.06	64.25	73.47	80.52	86.95	92.36	169.86	268.42	424.83	555.75	667.39	778.06	877.83
26	2010	148	22.81	28.67	36.07	41.25	45.21	48.82	51.86	22.87	36.12	57.24	74.78	89.86	104.84	118.26
27	2011	152	23.42	29.44	37.05	42.37	46.43	50.14	53.26	17.35	27.41	43.44	56.74	68.18	79.56	89.74
28	2012	109	16.80	21.12	26.57	30.38	33.30	35.96	38.19	116.47	184.01	291.44	380.95	457.66	533.80	602.18
29	2013	237	36.52	45.91	57.77	66.06	72.40	78.18	83.04	79.81	126.13	199.59	261.15	313.58	365.55	412.43
		Average	27.59	34.68	43.64	49.90	54.69	59.06	62.73	2653.68	4193.03	6638.50	8681.14	10426.91	12158.56	13716.92
		S								9.74	12.24	15.40	17.61	19.30	20.84	22.13
		k2	-0.16	-0.164	-0.16	-0.16	-0.16	-0.164	-0.164							
		pt	26	32.673	41.11	47	51.5	55.64	59.1001128							
		k5	0.72	0.719	0.719	0.72	0.72	0.719	0.719							
		PT	34.6	43.482	54.71	62.6	68.6	74.04	78.6451728							

Determined mean and standard deviation of each series of 5min, 10min, 15min, 20min60min.rainfall depth for each min is calculating by Gumbel extreme event formula for given return period.

Converted the depth into intensity of rainfall for 2 year return period

TIME DURATION	INENSITY
	2 YEAR
5min(0.083hr)	313.1738
10min(0.166hr)	196.8258
20min(0.333hr)	123.4678
30min(0.5hr)	94.02458
40min(0.66hr)	78.06852
50min(0.833hr)	66.79774
60min(1hr)	59.10011

Plotting of the intensity duration curve on log -log graph paper to determine the value of a and n constants

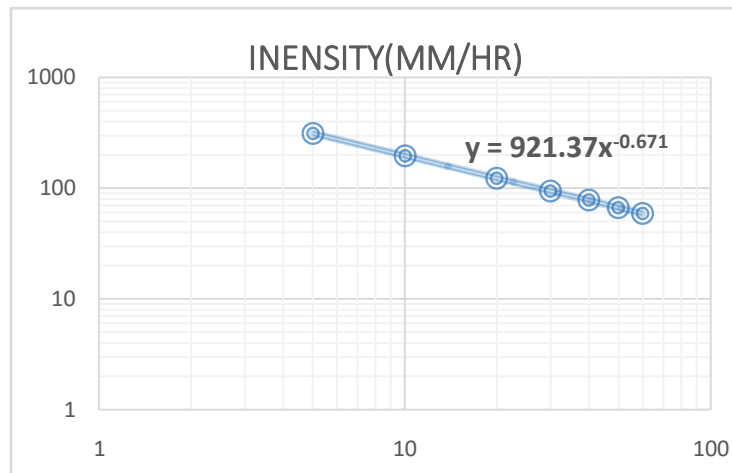
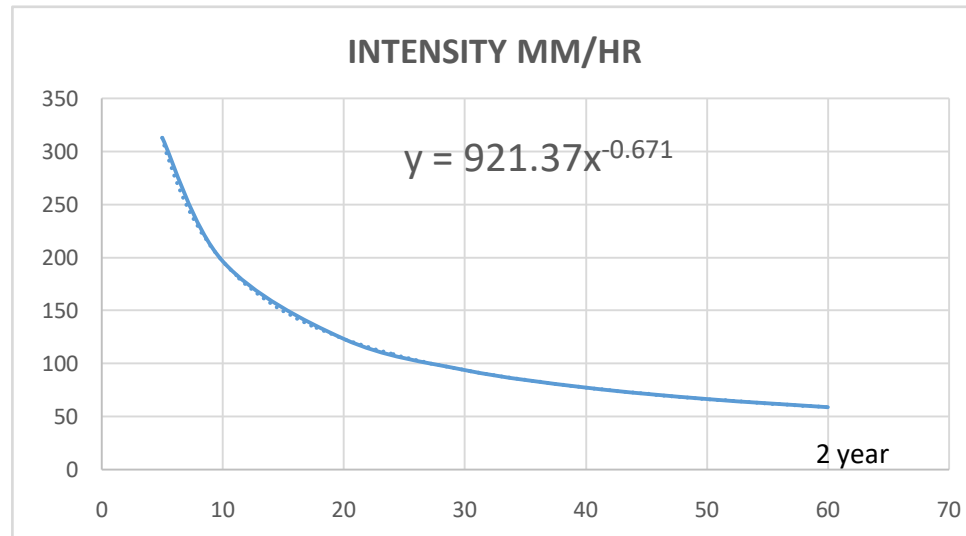


Fig. 4: log-log graph of intensity vs duration for 2year

Calculated rainfall intensity by $i = a/t^n$ for 5min 10min 20min.....60min for 2 year return period

TIME DURATION (MIN)	A	n	INTENSITY MM/HR
5	921	0.671	312.91
10	921	0.671	196.53
20	921	0.671	123.44
30	921	0.671	94.03
40	921	0.671	77.53
50	921	0.671	66.75
60	921	0.671	59.06

Plot of IDF curve for 2year return period



VII. Plot of IDF curve for 5 year return period

Converted the depth into intensity of rainfall for 5 year return period

Plotting of the intensity duration curve on log -log graph paper to determine the value of a and n constants

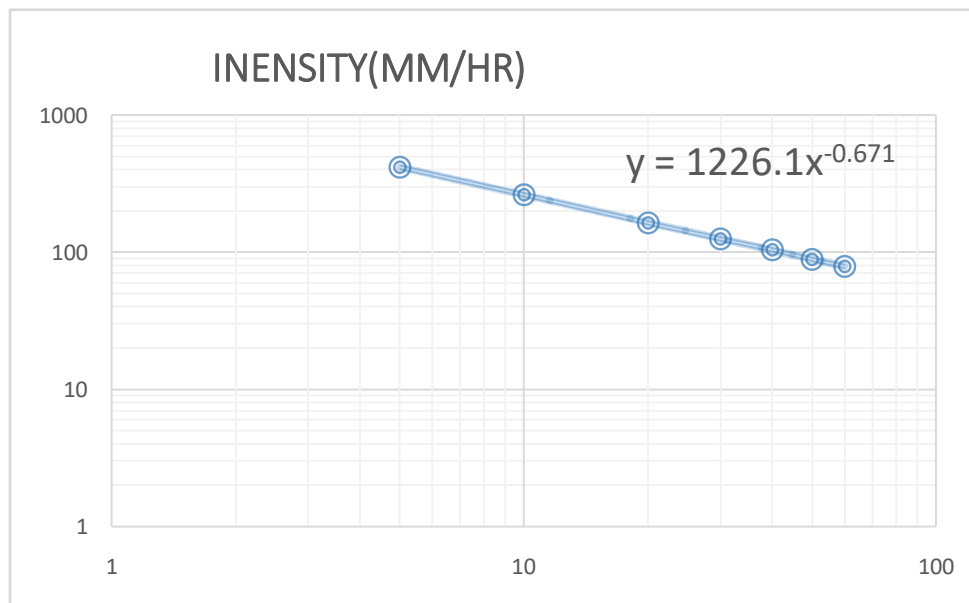


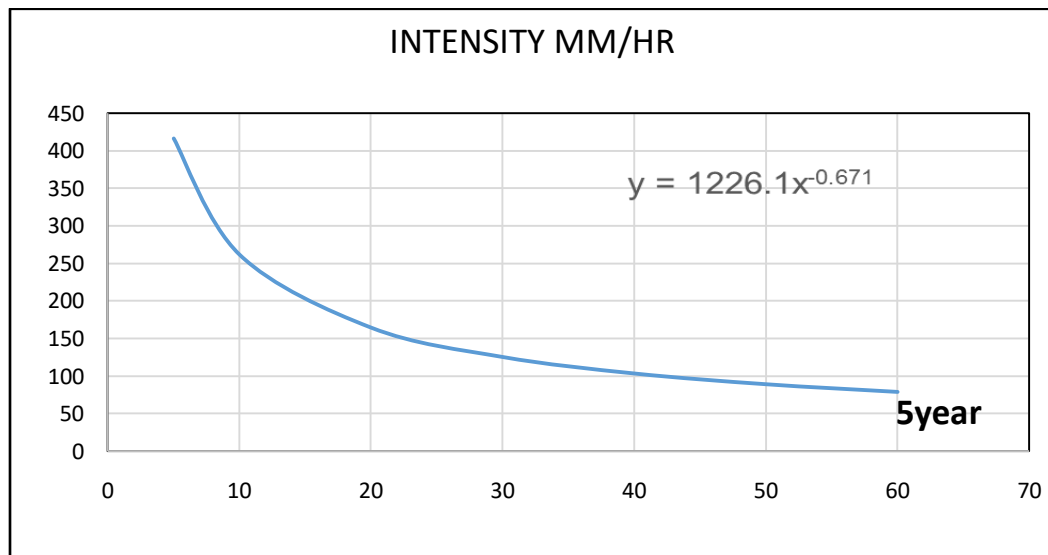
Fig. 4: log-log graph of intensity vs duration for 5year

Calculated rainfall intensity by $i = a/t^n$ for 5min 10min 20min.....60min for 5 year return period

TIME DURATION (MIN)	A	n	INTENSITY MM/HR
5	1226	0.671	416.41
10	1226	0.671	261.53
20	1226	0.671	164.26
30	1226	0.671	125.13
40	1226	0.671	103.17

50	1226	0.671	88.82
60	1226	0.671	78.59

Plot of IDF curve for 2year return period



VIII. Concluding Remarks

IDF is a tool for planning, design and operation of water Resources projects such as storm water drainage system. With the help of this tool (IDF) relationship of rainfall has been established at central zone of Surat city from daily/24 hours rainfall data using Gumbel distribution method. These relationships are useful in the design of urban drainage works, e.g. storm sewers, culverts and other hydraulic structures.

ACKNOWLEDGEMENT

The authors are thankful to the State Water Data Centre (SWDC), Gandhinagar for providing the rainfall data. The authors are also thankful to the Director and Faculty members of Bhaskaracharya Institute for Space Application and Geo-Informatics (BISAG), Gandhinagar for their support & guidance provided for this study.

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