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## Analysis of Reinforced Concrete Building Frame resting on different Support and Soil Condition with Consideration of Soil Structure Interaction

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**Abstract** — Present Analysis investigates the effect of soil structure interaction on the structural behavior of a building during an earthquake of G+2, G+6 & G+12 Storey Building. The analysis is done by using (Bentley STAAD Pro V8i Version 2007) software. The Analysis carried out for 3x3 bays with G+2, G+6 and G+12 storey's R.C.C Frame building with different Support Condition like Fixed, Hinged & Spring. The interactive analyses are carried out for 3 different values of modulus of sub grade reaction. Comparison of both nodal displacements and reactions of the frame is done for different soil and support conditions for all three cases.

Keywords—Building Analysis, Soil structure interaction, Support Condition, STAAD Pro V8i, Displacement.

#### I. INTRODUCTION

When structure is built on ground some elements of structure are direct contact with soil. When loads are applied on structure internal forces are developed in both the structure as well as in soil. It results in deformation of both the components which are independent to each other. This are called soil structure interaction. All problems in Civil Engineering involve interaction of structural elements with ground. When forces are applied externally to the structural element, the physics of the problem dictates the structural Element and ground to deform in a compatible manner. This is because of inherent intendancy of structural-element displacements and ground displacements by the virtue of their intimate physical contact. Therefore, these types of problems are broadly referred to as Soil-structure interaction (SSI) problems. In conventional structural design, SSI effects are not considered. Neglecting SSI effect for a relatively flexible structure founded on hard soil is reasonable. But, for a relatively stiff structure founded on either soft or medium soil neglecting SSI has a great impact on structural response and design. IS 1893(Part I):2002 suggests that SSI may not be considered in the seismic analysis of structure supported on rock or rock like material.

## II. LITERATURE REVIEW

Soil structure interactions have been studied by many researches. Musande et al.[1] concluded that Soil Structure Interaction more affected on fixed base. Venkatesh et al.[2] focused on the response spectrum analysis of the soil-structure model considering fixed base and flexible base of modeling the structure. Nirav et al.[3] focused on the effect of Soil-Structure Interaction on a six storey building loaded as per IS 1893. The comparison of various forces, storey drift, storey displacement, has been presented subjected to seismic force and supported on different types of soil condition. Meghna et al.[4] focused on analysis of space frame and considered linear springs having stiffness equal to the modulus of sub grade reaction of the soil. Present Analysis investigates the effect of soil structure interaction on the structural behavior of a building during an earthquake of G+2, G+6 & G+12 Storey Building.

#### III.OBJECTIVE OF THE RESEARCH WORK

- The objectives of the present investigations are to Study changes in Nodal Displacements and Reactions in case of different Support and Soil Condition.
- o Comparison of nodal displacements and reactions of the frame for different soil and support conditions.

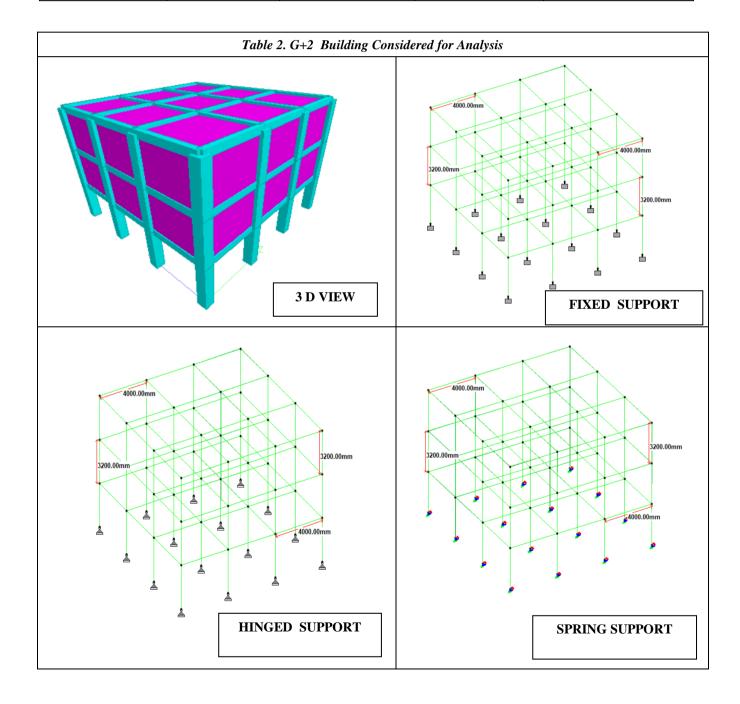
## IV. METHODOLOGY

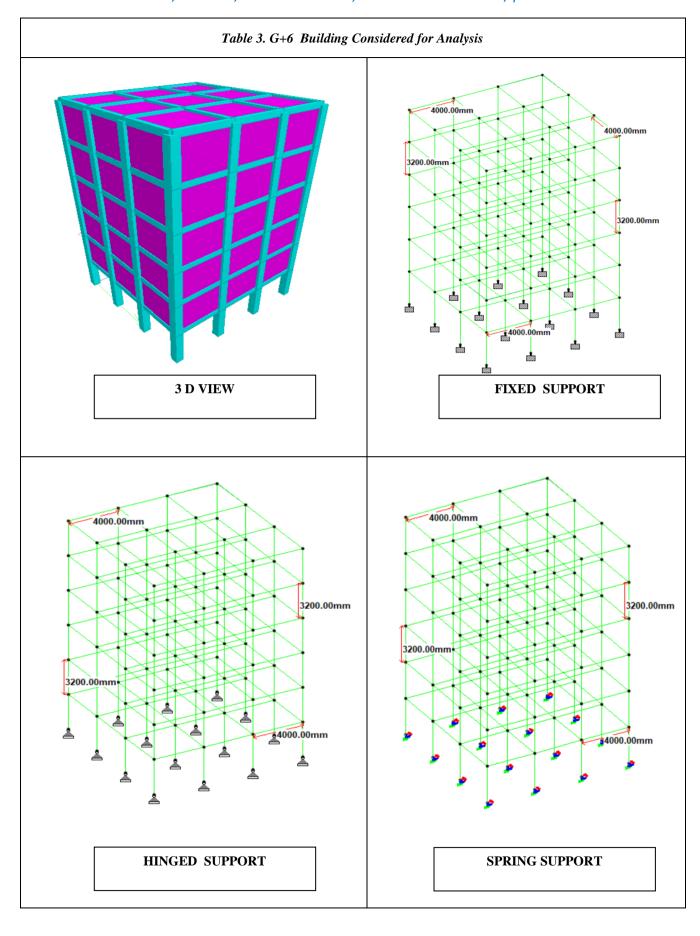
Hard, Medium and Soft Soil are the three types of soil upon which the structural frames are considered to be resting. Fixed, Hinged & Spring Support Conditions are considered in the analysis for G+2, G+6 & G+12 Storey Building. Total 27 structures are analyzed as presented in table 1.

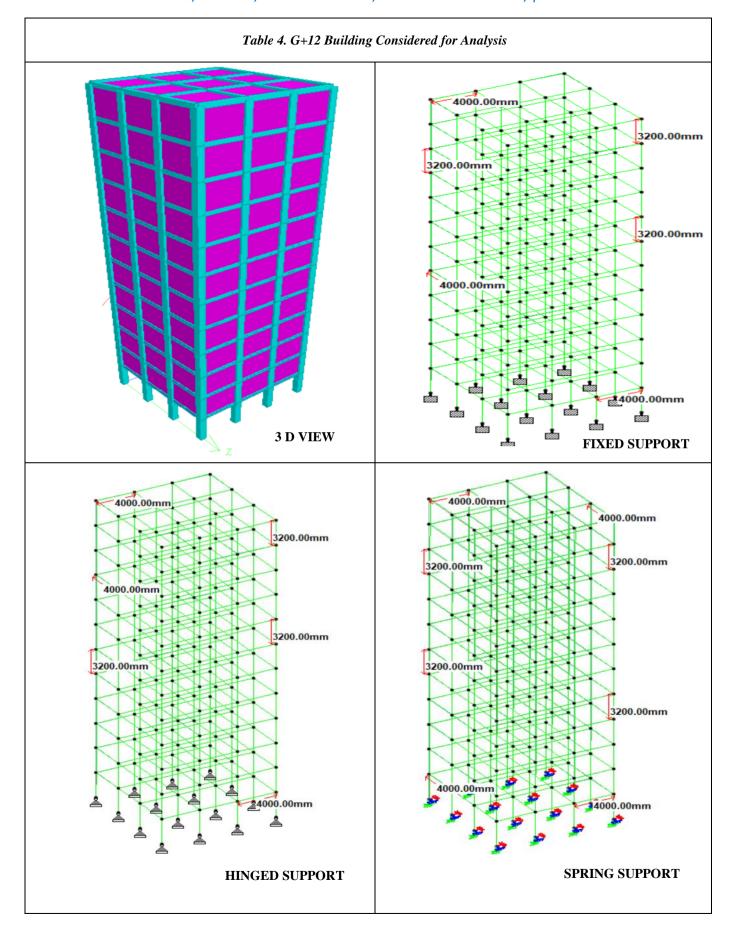
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Table 1.Types of Building Considered For Analysis

BUILDING	SOIL TYPE	TYPE OF SUPPORT						
	SOFT	FIXED	HINGED	SPRING				
G + 2	MEDIUM	FIXED	HINGED	SPRING				
	HARD	FIXED	HINGED	SPRING				
	SOFT	FIXED	HINGED	SPRING				
G + 6	MEDIUM	FIXED	HINGED	SPRING				
	HARD	FIXED	HINGED	SPRING				
	SOFT	FIXED	HINGED	SPRING				
G + 12	MEDIUM	FIXED	HINGED	SPRING				
	HARD	FIXED	HINGED	SPRING				







#### V. DESCRIPTION OF THE BUILDING

In the present analysis a 3x3 bays with G+2, G+6 and G+12 storey R.C. Frame building is considered to investigate SSI effects on tall buildings. The plan dimension of the building is 12.0 m x 12.0 m and the height of the building is 9.6 m, 19.2 m, and 38.4m from the ground level for G+2, G+6 and G+ 12 storey building respectively.

Table 5.Discription of Building

Component	Description	Data
Frame	No. of Storey	G+2, G+ 6 and G+12
	No. of bays in X and Z direction	3 x 3 bays
	Storey height	3.2m
	Bay width in X and Z direction	4m x 4m
	Size of beam	300mm x 450mm
	Size of column	300mm x 500mm
	Thickness of slab	125 mm

#### VI. SEISMIC PARAMETERS

For the SSI analysis using Bureau of Indian Standards in IS 1893 (Part I): 2002 is used for Static and Dynamic analysis. The building is assumed to be situated in Zone IV. Medium, hard and soft are three types of soil upon which structural frames are considered to be resting. The following table is about Seismic Parameters.

Table.6 Seismic Parameters

Table 7.Rock and Soil Site Factor (SS)

Sr. No.	Parameters	Values
1	Zone ( IV)	0.24
2	Response Reduction Factor (RF) (SMRF)	5
3	Rock and Soil Site Factor (SS)	1
4	Type of Structure ( RC frame building)	1
5	Damping Ratio (DM)	5
6	Period in x Direction (PX)	0.5
7	Period in z direction (PZ)	0.5
8	Importance factor ( I) ( Important Building)	1.5
9	Depth of Foundation (DT)	3 m

ı	S.No	Type of Soil	Value
ĺ	1	Soft	3
ĺ	2	Medium	2
	3	Hard	1

#### VII. SOIL MODELING AND ANALYSIS

#### 7.1 Soil Modeling

In Reinforced Concrete building with shallow foundation resting on three different soils (soft, medium and hard), Each Soil has six soil springs, whose stiffness's are calculated From Table 9. These soil springs represent the stiffness of soil in three translational directions and three rotational directions.  $K_x$ ,  $K_y$ ,  $K_z$  are translational soil stiffness's in kN/m in x,y and z directions respectively.  $K_{\Theta x}$ ,  $K_{\Theta y}$ ,  $K_{\Theta z}$  are rotational spring stiffness's in kN-m/rad about x, y and z directions respectively.

Table 8. Characteristic Properties Of Soils

Type of soil	Shear wave velocity Vs (m/s)	Elastic modulus E (kg/cm <sup>2</sup> )	Shear modulus G (kg/cm <sup>2</sup> )	Density of soil ρ (kN/m³)	Poisson's ratio of soil
Hard	600	16400	6480	17.322	0.28
Medium	320	4945	1808	16.841	0.39
Soft	150	935	335	14.435	0.40

Table 9. Stiffness's of equivalent soil springs along various degrees of freedom

Degrees of freedom	Stiffness of equivalent soil spring
Translation along x – axis (Kx)	KZ -{[(0.2GL)/(0.75-μ)][1-(B/L)}
Translation along y – axis (Ky)	$\{[(2GL)/(1-\mu)][0.73+1.54(B/L)^{0.75}]\}$
Translation along z – axis (Kz)	$\{[(2GL)/(2-\mu)][2+2.5(B/L)^{0.85}]\}$
Rocking about x – axis (Krx)	$\{[(GI_X^{0.75})/(1-\mu)](L/B)^{0.25}[2.4+0.5(B/L)]\}$
Torsion along y – axis (Kry)	$GJ_t^{0.75}$ {4+11[1-(B/L)] <sup>10</sup> }
Rocking about z – axis (Krz)	$\{[(GI_z^{0.75})/(1-\mu)][3(L/B)^{0.15}]\}$

To compute the soil-spring stiffness, the effective shear modulus, Poisson's ratio and dimensions of Foundation are required. The foundations are designed as per code provisions by obtaining the design forces from Staad Pro. In present study to calculate stiffness form above formulae following data is used. L=2m, B=1.5m.

Table 10. Stiffness's Constant Value of different types of soil springs

Direction	Unit	Soft Soil	Medium Soil	Hard Soil
Vertical, K <sub>z</sub>	kN/m	8,679.07	45,897.75	644,502.27
Horizontal, K <sub>x</sub>	kN/m	5,786.05	36,936.76	5,80,052.24
Horizontal, K <sub>Y</sub>	kN/m	5,786.05	36,936.76	5,80,052.24
Rocking, $K_{\Theta x}$	kN-m/rad	14,72,678.0	77,87,998.16	10,93,60,086.95
Rocking , $K_{\Theta y}$	kN-m/rad	4,25,510.46	22,50,237.11	3,15,98,123.26
Twisting , $K_{\Theta z}$	kN-m/rad	9,98,328.62	68,63,330.52	11,12,02,826.79

## 7.2 Analysis Data:

1) Live Load : 3.0 kN/m² at typical floor

: 1.5 kN/m<sup>2</sup> on terrace

2) Floor finish : 1.0 kN/m<sup>2</sup>

3) Earthquake Load : As per IS-1893(Part 1)-2002

Using STAAD Program.

4) Depth of Foundation : 3 m 5) Storey Height : 3.2 m

6) Walls : 230 mm thick brick masonry wall

7) Compressive strength : 20 N/mm<sup>2</sup>

of Concrete (fck)

8) Reinforcement ( $f_y$ ) : 415 N/mm<sup>2</sup> 9) Poisson's ratio( $\mu$ ) : 0.15

#### VIII. RESULTS AND DISCUSSIONS

The results are presented in the form of tables and graphs considering the effect of different Support and Soil Condition. The variation of Nodal Displacement and Reaction of the building models resting on different types of soil and Support are presented. The properties of the soil used for present study are given in tables 8. The trends observed in the results are also discussed in these sections.

## 8.1 Comparison of Reaction at Node in different Soil - G+2, G+6,G+12 Building

The value of node reactions are presented in the table11, 12 and 13 for G+2, G+6,G+12 buildings respectively. Chart-1 shows the variation of Moment at Z direction of node for fixed and spring support conditions and different Soil conditions. From the chart it is clear that in case of soft soil with spring support its moment value is quit high compared to fixed support.

Table 11. Reaction at Node 1 - G+2 Building (EQ X)

SUPPORT	SOIL TYPE			Reaction In KN			Moment In KN-m			
		Node	L/C	V <sub>X</sub>	$\mathbf{V}_{\mathbf{Y}}$	$\mathbf{V}_{\mathbf{Z}}$	M <sub>X</sub>	M <sub>Y</sub>	$M_{\rm Z}$	
	SOFT	1	1 EQ X	-32.449	-62.131	-0.674	-0.797	-0.127	56.324	
FIXED	MEDIUM	1	1 EQ X	-32.449	-62.131	-0.674	-0.797	-0.127	56.324	
	HARD	1	1 EQ X	-31.731	-60.758	-0.659	-0.779	-0.125	55.079	
	SOFT	1	1 EQ X	-32.415	-80.121	-0.305	0.000	0.000	0.000	
HINGED	MEDIUM	1	1 EQ X	-32.415	-80.121	-0.305	0.000	0.000	0.000	
	HARD	1	1 EQ X	-31.698	-78.350	-0.298	0.000	0.000	0.000	
	SOFT	1	1 EQ X	-33.388	-39.128	-0.091	0.254	-0.145	134.073	
SPRING	MEDIUM	1	1 EQ X	-33.112	-56.935	-0.256	-0.078	-0.147	74.825	
	HARD	1	1 EQ X	-31.841	-60.419	-0.599	-0.679	-0.133	56.436	

Table 12. Reaction at Node 1 - G+6 Building (EQ X)

SUPPORT	SOIL TYPE				Reaction In KN			Moment In KN-m		
		Node	L/C	V <sub>X</sub>	$\mathbf{V}_{\mathbf{Y}}$	$\mathbf{V}_{\mathbf{Z}}$	M <sub>X</sub>	$M_{Y}$	$M_{Z}$	
	SOFT	1	1 EQ X	-71.406	-274.419	-4.335	-4.892	-0.270	123.689	
FIXED	MEDIUM	1	1 EQ X	-58.151	-223.479	-3.530	-3.984	-0.220	100.729	
	HARD	1	1 EQ X	-42.758	-164.323	-2.596	-2.929	-0.161	74.065	
	SOFT	1	1 EQ X	-70.637	-313.147	-2.178	0.000	0.000	0.000	
HINGED	MEDIUM	1	1 EQ X	-57.525	-255.018	-1.774	0.000	0.000	0.000	
	HARD	1	1 EQ X	-46.159	-205.807	-1.517	0.000	0.000	0.000	
	SOFT	1	1 EQ X	-72.358	-179.114	-0.585	1.854	-0.302	474.536	
SPRING	MEDIUM	1	1 EQ X	-58.896	-212.198	-1.366	-0.205	-0.265	166.867	
	HARD	1	1 EQ X	-42.902	-167.876	-2.400	-2.573	-0.175	77.573	

Table 13. Reaction at Node 1 - G+12 Building (EQ X)

SUPPORT	SOIL TYPE				Reaction In KN			Moment In KN-m		
		Node	L/C	V <sub>X</sub>	$\mathbf{V}_{\mathbf{Y}}$	$V_{\rm Z}$	M <sub>X</sub>	M <sub>Y</sub>	$M_{\rm Z}$	
	SOFT	1	1 EQ X	-92.662	-697.292	-13.200	-14.797	-0.369	159.901	
FIXED	MEDIUM	1	1 EQ X	-75.462	-567.854	-10.750	-12.050	-0.301	130.218	
	HARD	1	1 EQ X	-55.486	-417.540	-7.904	-8.860	-0.221	95.749	
	SOFT	1	1 EQ X	-91.916	-761.680	-6.842	0.000	0.000	0.000	
HINGED	MEDIUM	1	1 EQ X	-73.300	-607.115	-5.535	0.000	0.000	0.000	
	HARD	1	1 EQ X	-53.897	-446.408	-4.070	0.000	0.000	0.000	
	SOFT	1	1 EQ X	-91.262	-465.111	-1.785	5.728	-0.416	1064.669	
SPRING	MEDIUM	1	1 EQ X	-75.399	-550.825	-4.201	-0.548	-0.399	298.592	
	HARD	1	1 EQ X	-55.679	-433.504	-7.374	-7.847	-0.252	104.524	

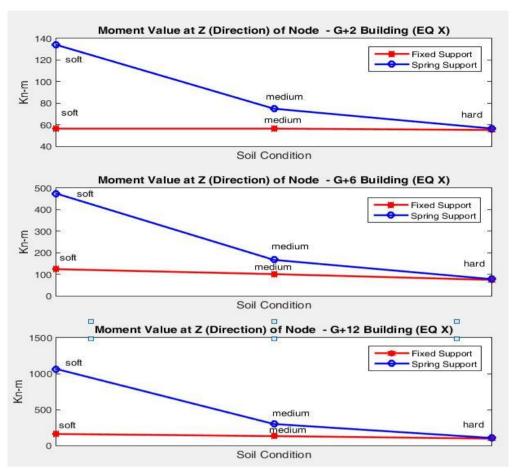


Chart 1. Moment Value at Z (Direction) of different Building

## 8.2 Comparison of Displacement at Node in different Soil – G+2, G+6,G+12 Building

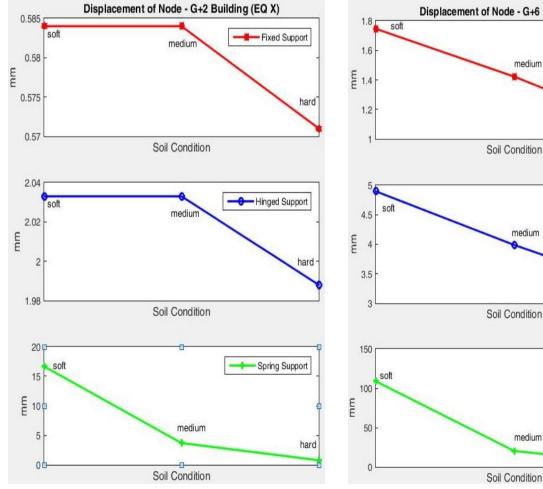
Table14. Displacement at Node - G+2 Building (EQ X)

SUPPORT	SOIL TYPE			Horizontal	Vertical	Horizontal	Resultant	ROTATIONAL		
		Node	L/C	X In mm	Y In mm	Z In mm	In mm	Θ <sub>X</sub> rad	Θ <sub>Y</sub> rad	Θ <sub>z</sub> rad
FIXED	SOFT	13	EQ X	0.583	0.032	0.001	0.584	0.000	0.000	-0.000
	MEDIUM	13	EQ X	0.583	0.032	0.001	0.584	0.000	0.000	-0.000
	HARD	13	EQ X	0.570	0.031	0.001	0.571	0.000	0.000	-0.000
	SOFT	13	EQ X	2.033	0.038	0.001	2.033	0.000	0.000	-0.000
HINGED	MEDIUM	13	EQ X	2.033	0.038	0.001	2.033	0.000	0.000	-0.000
	HARD	13	EQ X	1.988	0.037	0.001	1.988	0.000	0.000	-0.000
	SOFT	13	EQ X	15.228	6.787	0.001	16.672	0.000	-0.000	-0.001
SPRING	MEDIUM	13	EQ X	3.334	1.571	0.001	3.686	0.000	-0.000	-0.000
	HARD	13	EQ X	0.758	0.135	0.001	0.770	0.000	0.000	-0.000

The Displacement Value at Node is presented in the table 14, 15 and 16 for G+2, G+6, G+12 buildings respectively.

Table 15. Displacement at Node - G+6 Building (EQ X)

SUPPORT	SOIL TYPE			Horizontal	Vertical	Horizontal	Resultant	Resultant ROTATIONAL		
		Node	L/C	X In mm	Y In mm	Z In mm	In mm	Θ <sub>X</sub> rad	Θ <sub>Y</sub> rad	Θ <sub>Z</sub> rad
FIXED	SOFT	13	EQ X	1.736	0.183	0.002	1.746	0.000	-0.000	-0.000
1222	MEDIUM	<b>DIUM</b> 13	EQ X	1.414	0.149	0.002	1.422	0.000	-0.000	-0.000
	HARD	13	EQ X	1.040	0.110	0.001	1.046	0.000	-0.000	-0.000
	SOFT	13	EQ X	4.893	0.197	0.002	4.897	0.000	-0.000	-0.000
HINGED	MEDIUM	13	EQ X	3.985	0.161	0.002	3.988	0.000	-0.000	-0.000
	HARD	13	EQ X	3.172	0.130	0.001	3.175	0.000	-0.000	-0.000
	SOFT	13	EQ X	104.314	31.113	0.002	108.855	0.000	-0.000	-0.005
SPRING	MEDIUM	13	EQ X	19.432	5.894	0.002	20.306	0.000	-0.000	-0.001
	HARD	13	EQ X	1.959	0.402	0.001	1.999	0.000	-0.000	-0.000



Displacement of Node - G+6 Building (EQ X) Fixed Support medium hard: Soil Condition Hinged Support medium hard Soil Condition Spring Support medium hard

Chart 2.Displacement of Node- G+2 Building(EQ X)

Chart 3.Displacement of Node- G+6 Building(EQ X)

Displacement of Node is represented graphically in chart 2 and 3 for G+2 and G+6 Building respectively. From the chart it is clear that in case of soft soil with spring support its displacement value is quit high compared to fixed support .

Table 16. Displacement at Node - G+12 Building (EQ X)

SUPPORT	SOIL TYPE			Horizontal	Vertical	Horizontal	Resultant	ROTATIONAL		
FIXED		Node	L/C	X In mm	Y In mm	Z In mm	In mm	Θ <sub>X</sub> rad	Θ <sub>Y</sub> rad	Θ <sub>Z</sub> rad
	SOFT	13	EQ X	5.279	0.677	0.002	5.322	0	0	-0.000
	MEDIUM	13	EQ X	4.299	0.552	0.002	4.334	0	0	-0.000
	HARD	13	EQ X	3.161	0.406	0.001	3.187	0	0	-0.000
HINGED	SOFT	13	EQ X	9.499	0.711	0.002	9.525	0	0	-0.000
	MEDIUM	13	EQ X	7.590	0.567	0.002	7.611	0	0	-0.000
	HARD	13	EQ X	5.581	0.417	0.001	5.597	0	0	-0.000
SPRING	SOFT	13	EQ X	511.960	81.005	0.002	518.329	0	0	-0.014
	MEDIUM	13	EQ X	97.125	15.473	0.002	98.350	0	0	-0.003
	HARD	13	EQ X	7.889	1.165	0.001	7.974	0	0	-0.000

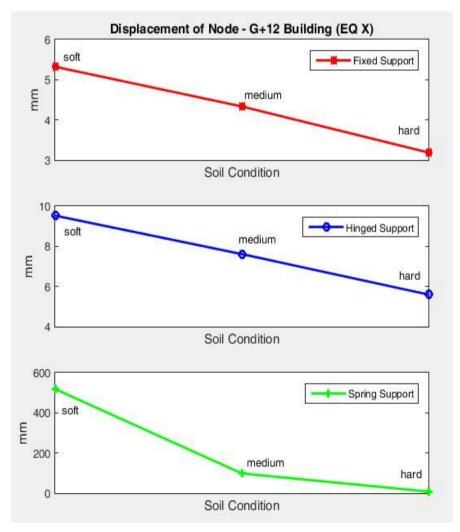


Chart 4. Displacement of Node- G+12 Building (EQ X)

Displacement at Node is presented in table 16 for G+12 Building. Displacement variation is plotted in Chart 4 for different support and soil condition. From the chart it is clear that in case of soft soil with spring support its displacement value is quit high compared to fixed support.

#### IX. CONCLUSION

Present Analysis investigates the effect of soil structure interaction on the structural behavior of a building during an earthquake of G+2, G+6 and G+12 Storey Building. Following observations were made during the analysis.

- 1. Moment value's along Z direction was high as compared with Fixed and Spring Support, Soil Type Soft and Medium.
- 2. Displacement at Node Increases exponentially as the Storey Height increases in case of Medium and soft soil -Spring Support.
- 3. For high rise building resting on soft, medium Soil. Soil Structure Interaction has to be considered in the analysis for safe design.
- 4. Structure built in location's which belongs to earthquake zone 3, 4 and 5. Soil Structure Interaction Should be considered in the analysis for safe design.

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